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2023 DRAINAGE IMPROVEMENTS

Conditions of Use

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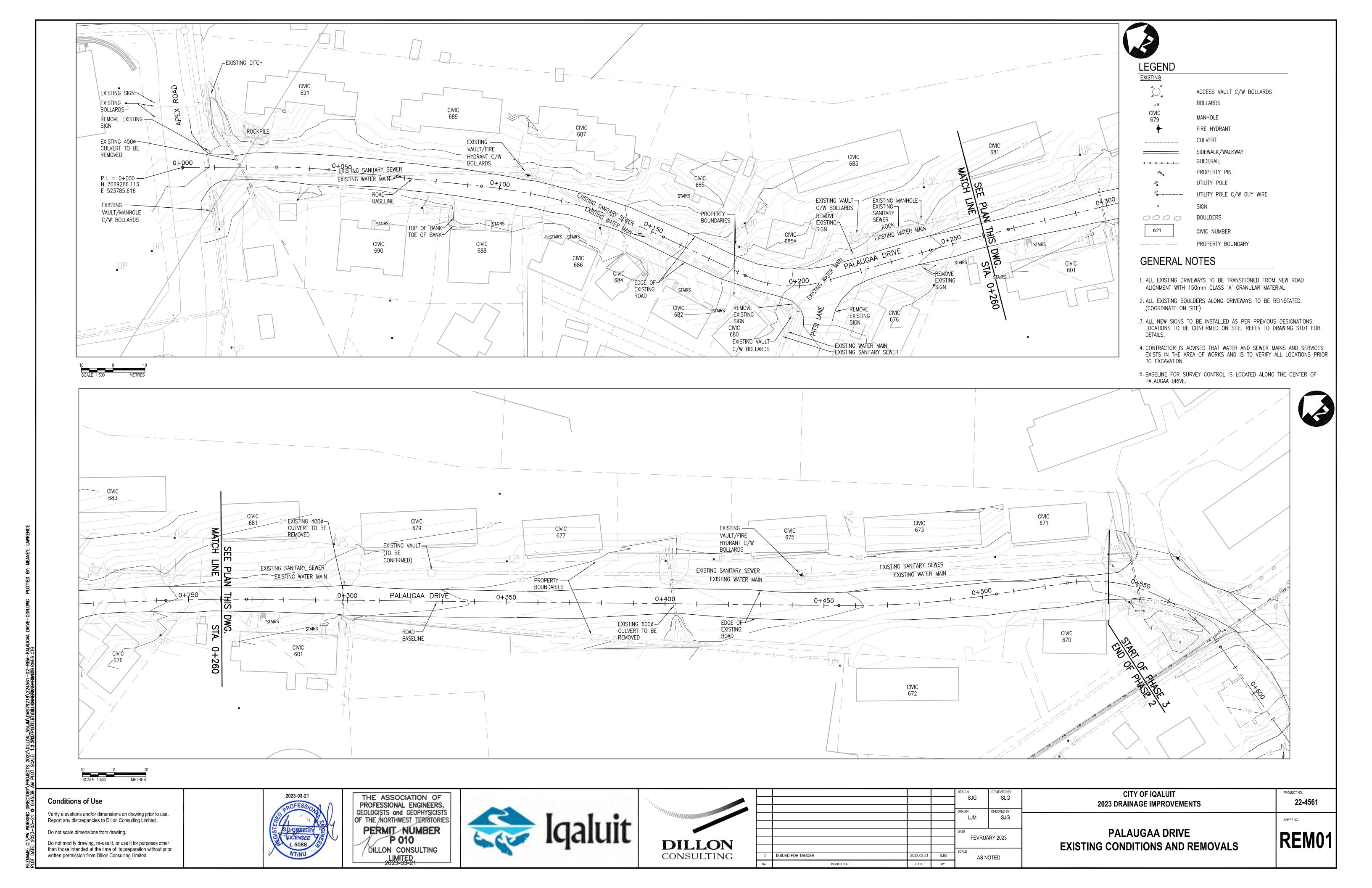
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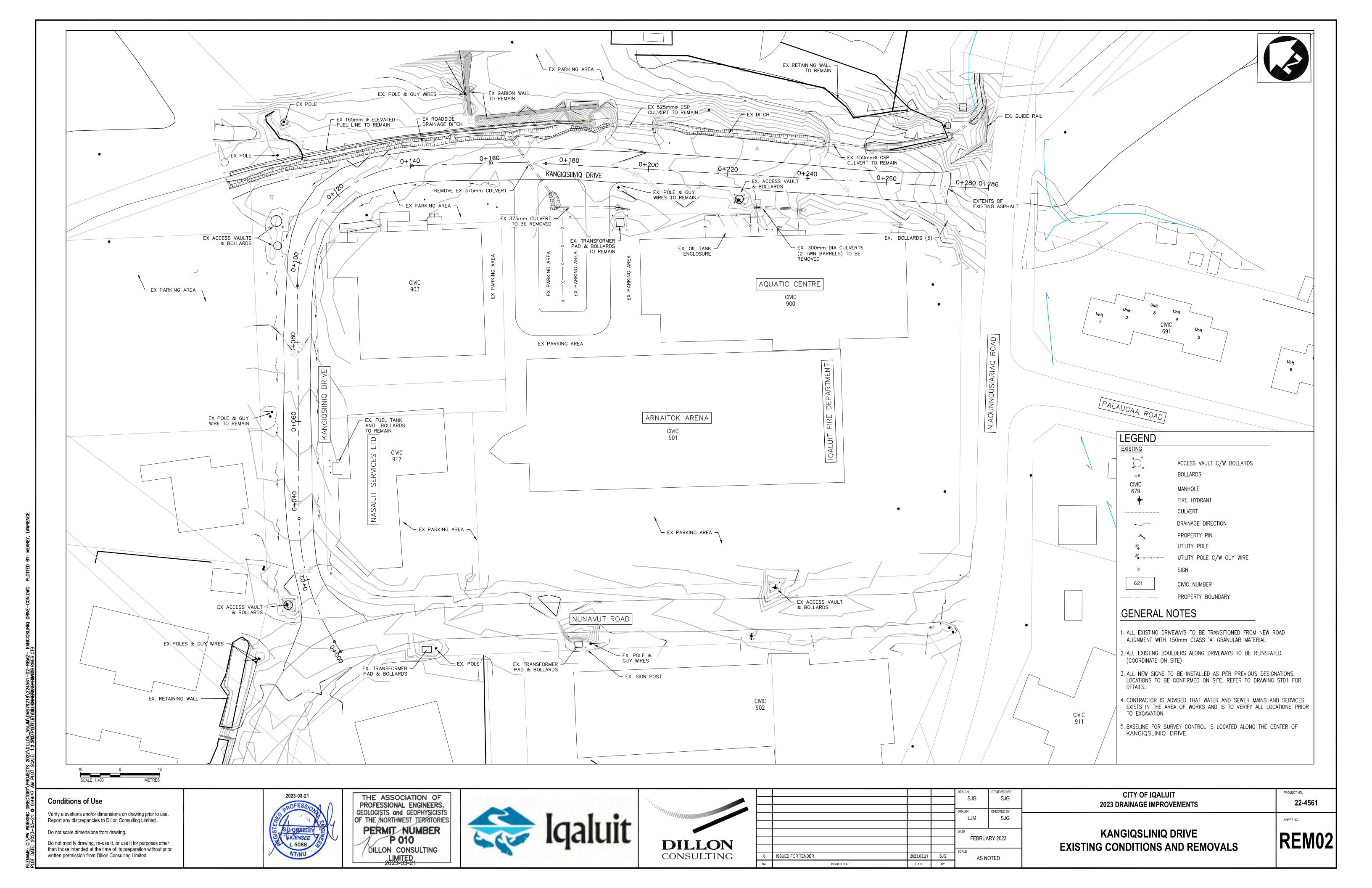
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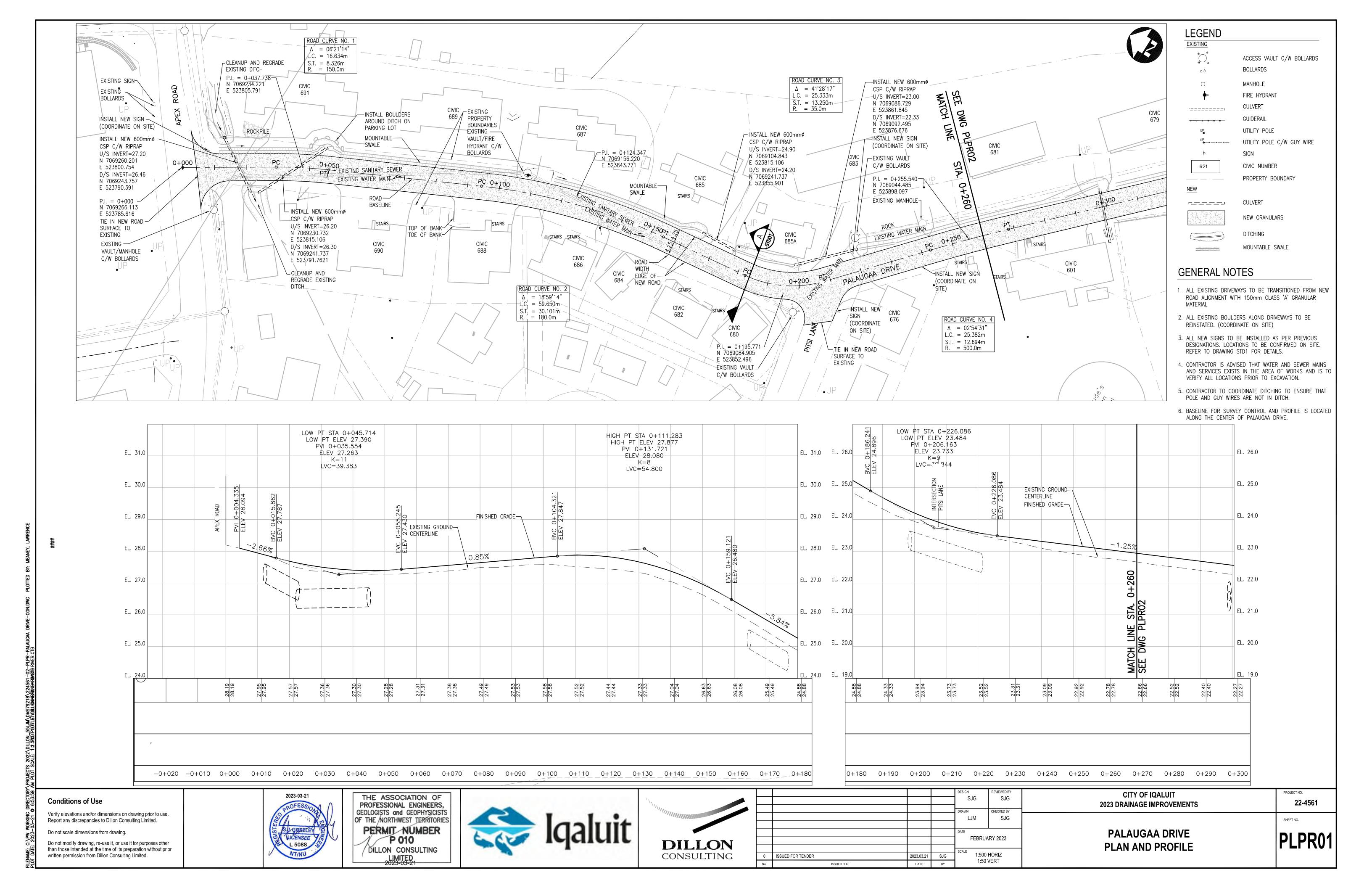
CITY PROJECT: 2023-RFT-056 DILLON PROJECT: 22-4561 DATE: MARCH 21, 2023 ISSUED FOR TENDER

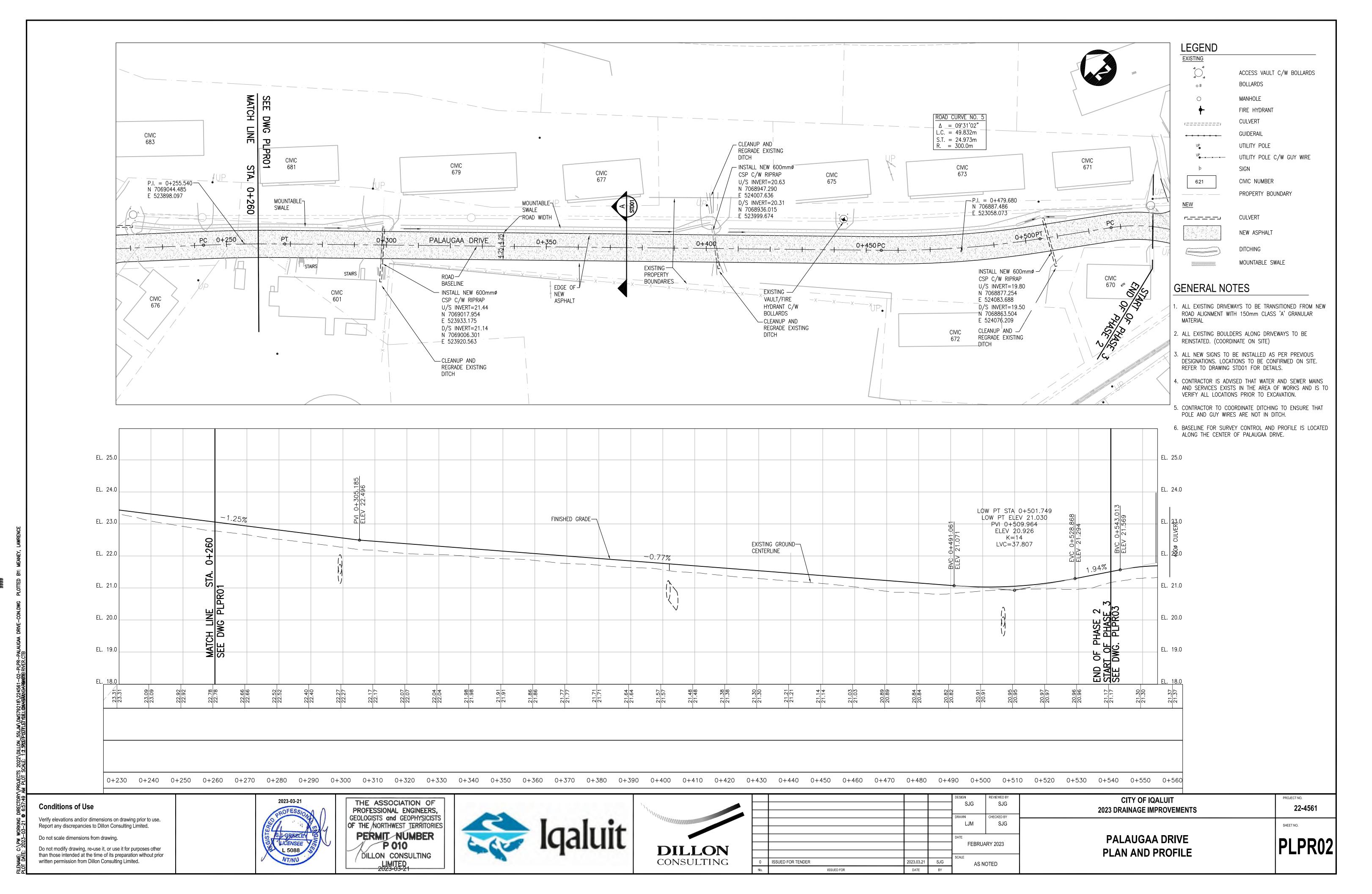
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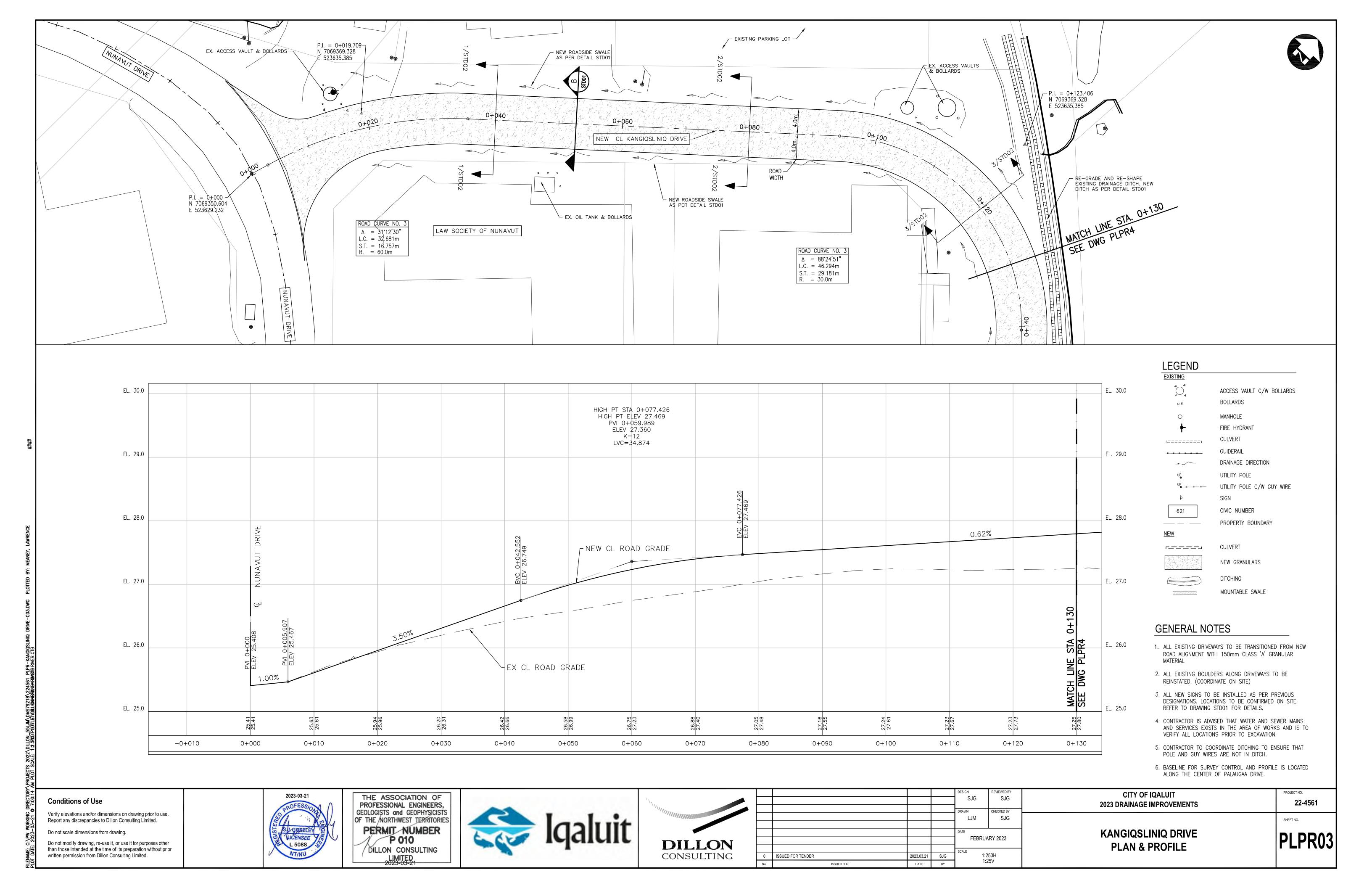
CONSULTING

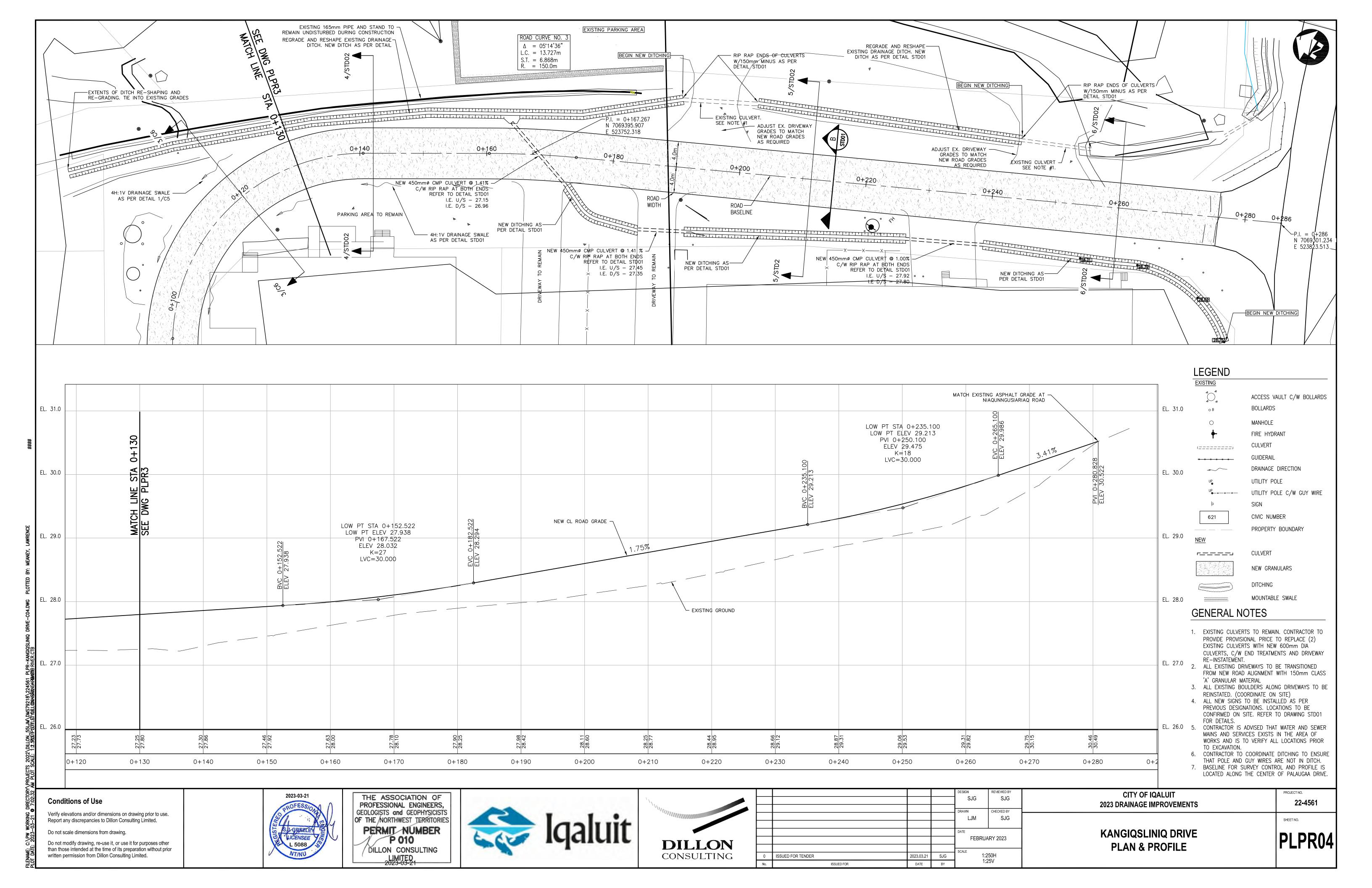


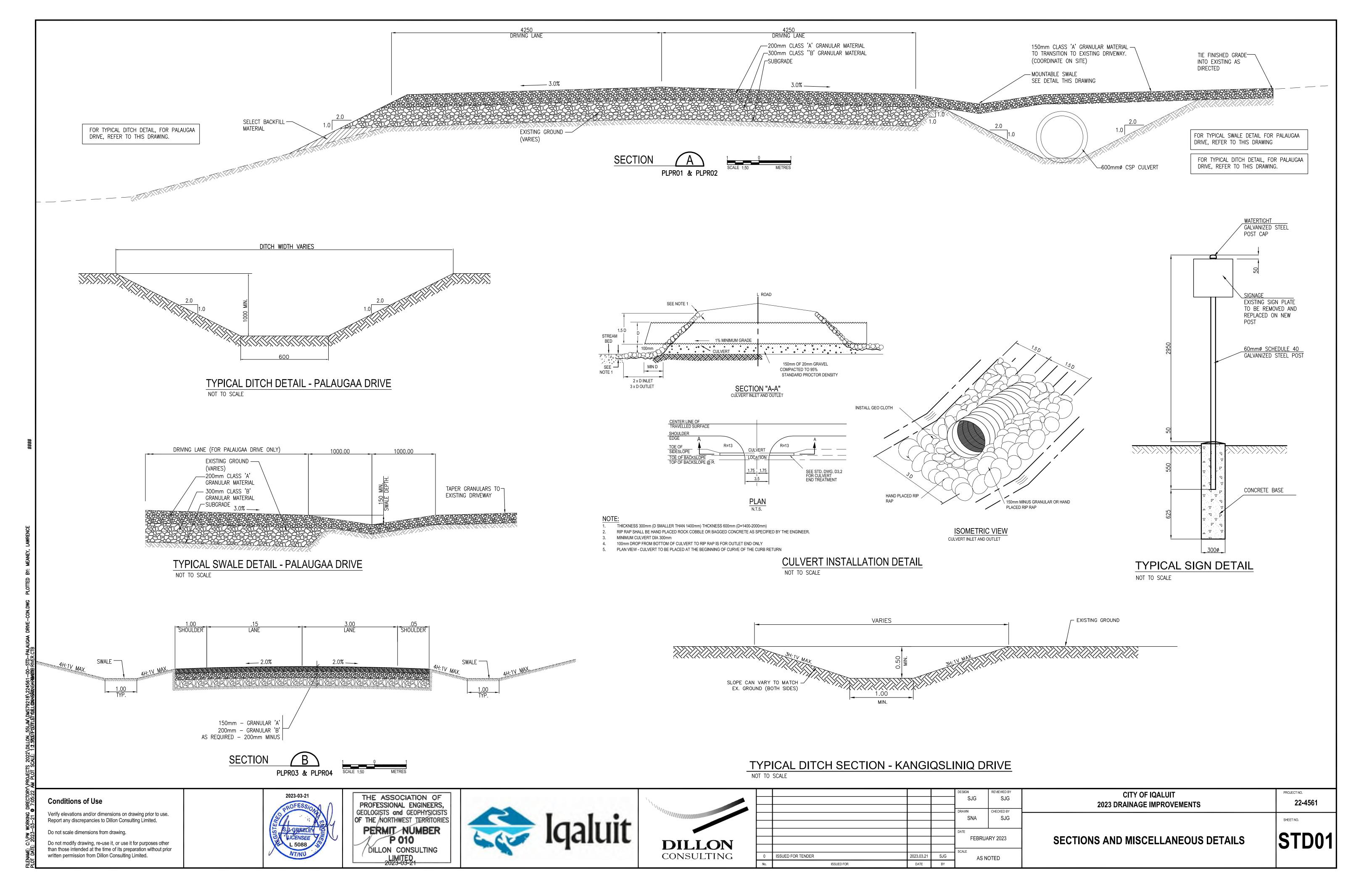




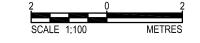








CROSS SECTIONS - KANGIQSLINIQ

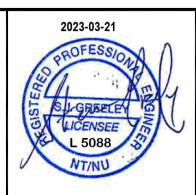


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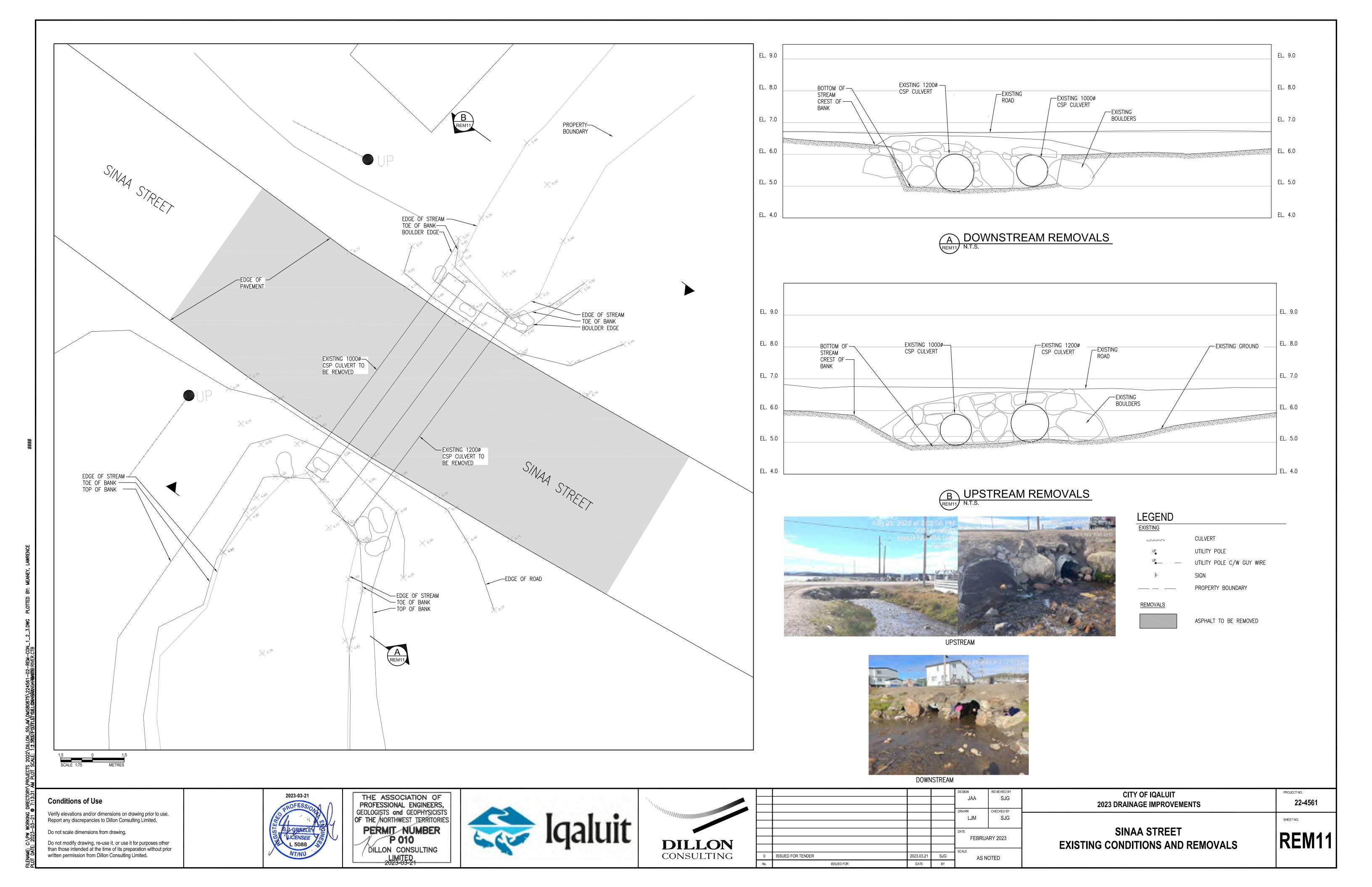
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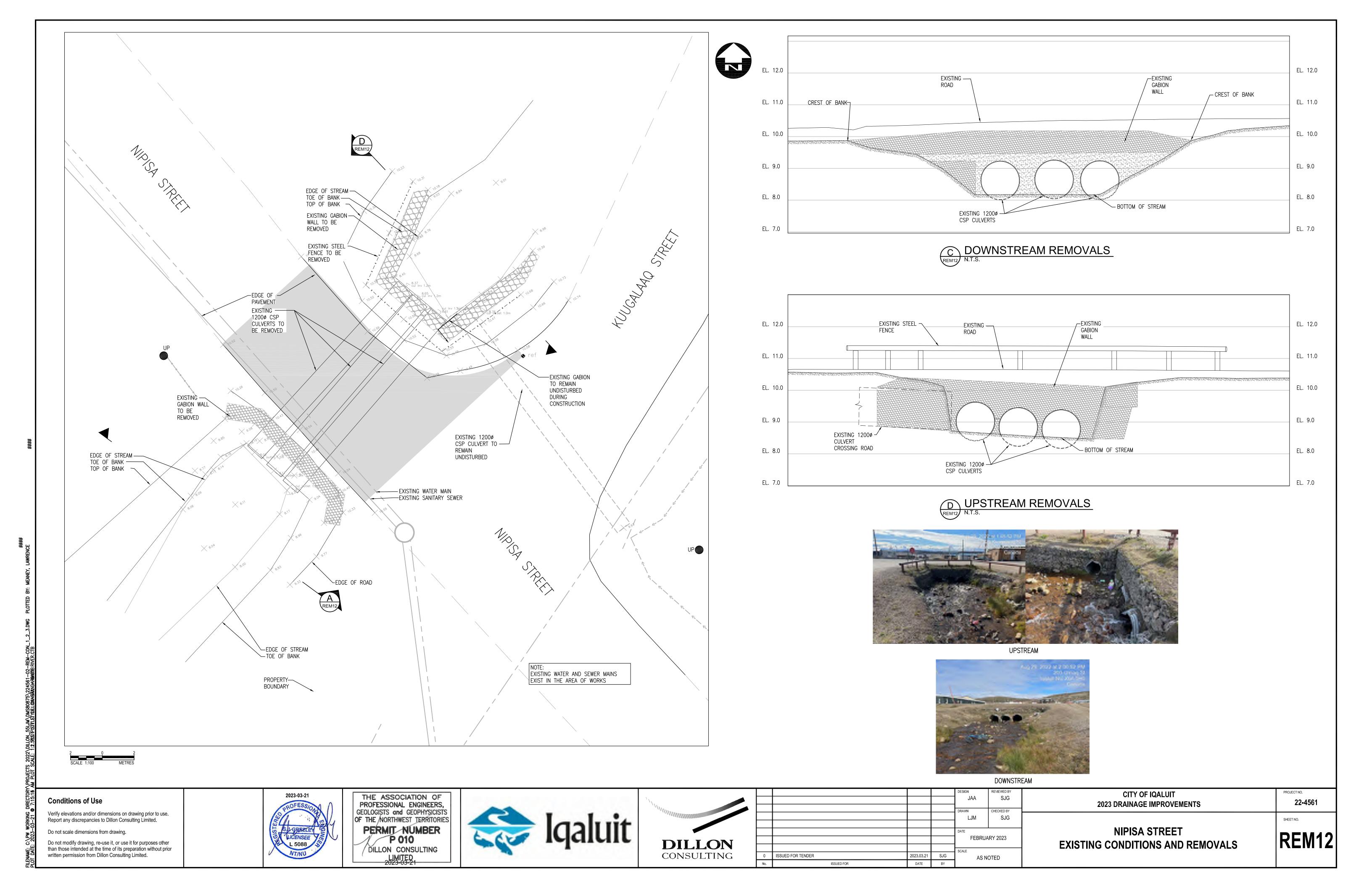
SECTIONS AND MISCELLANEOUS DETAILS

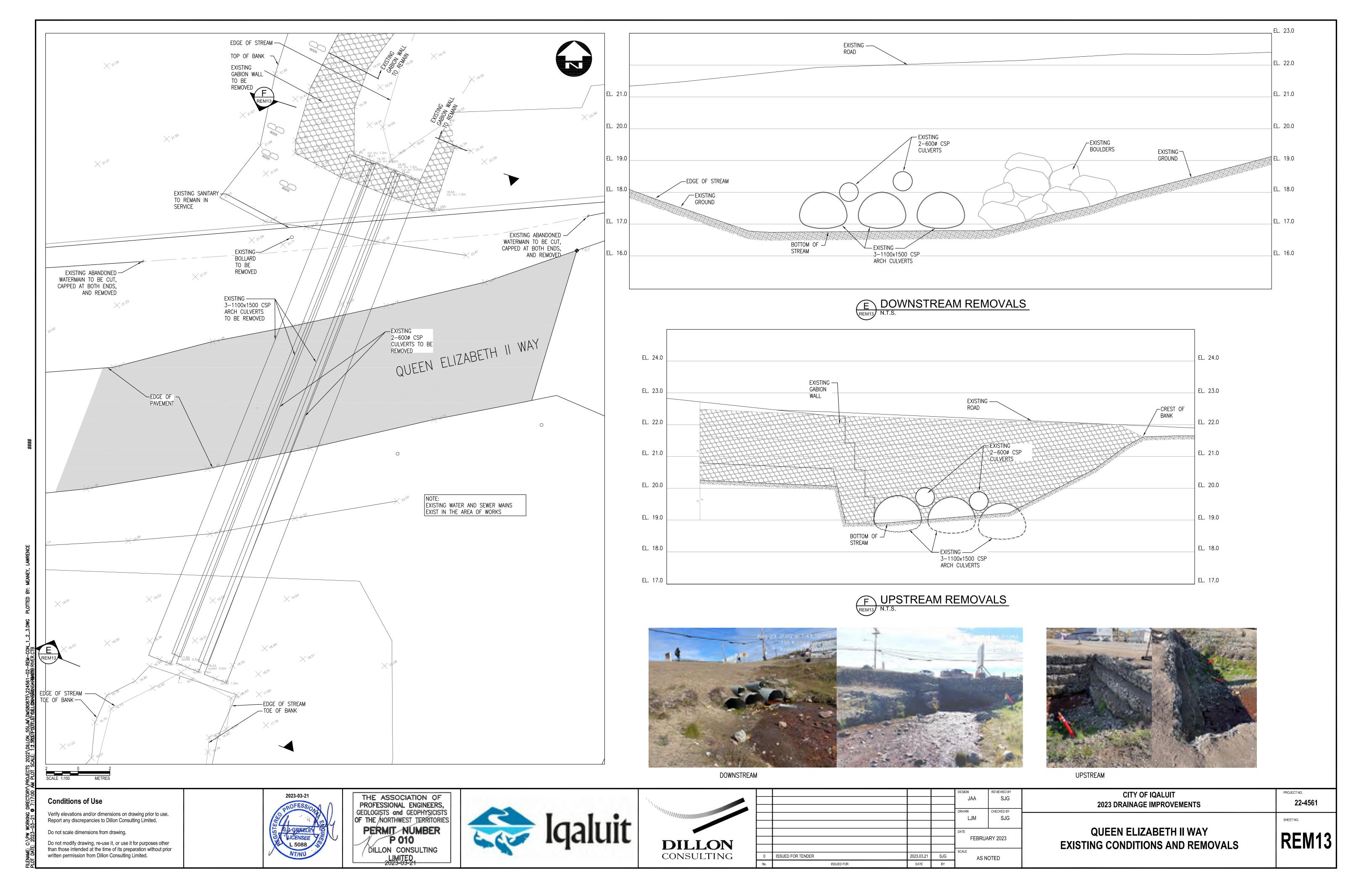
CITY OF IQALUIT

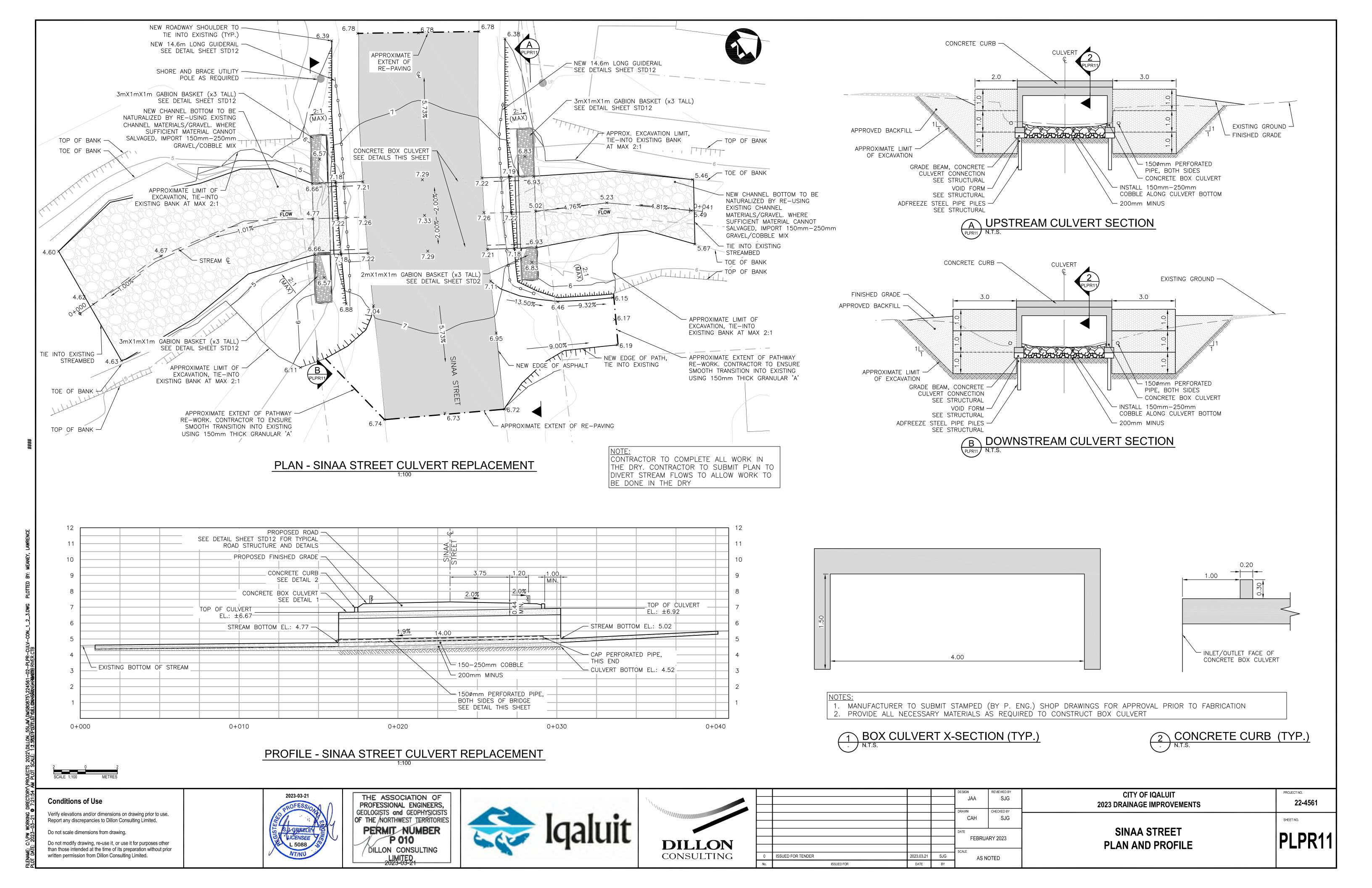
2023 DRAINAGE IMPROVEMENTS

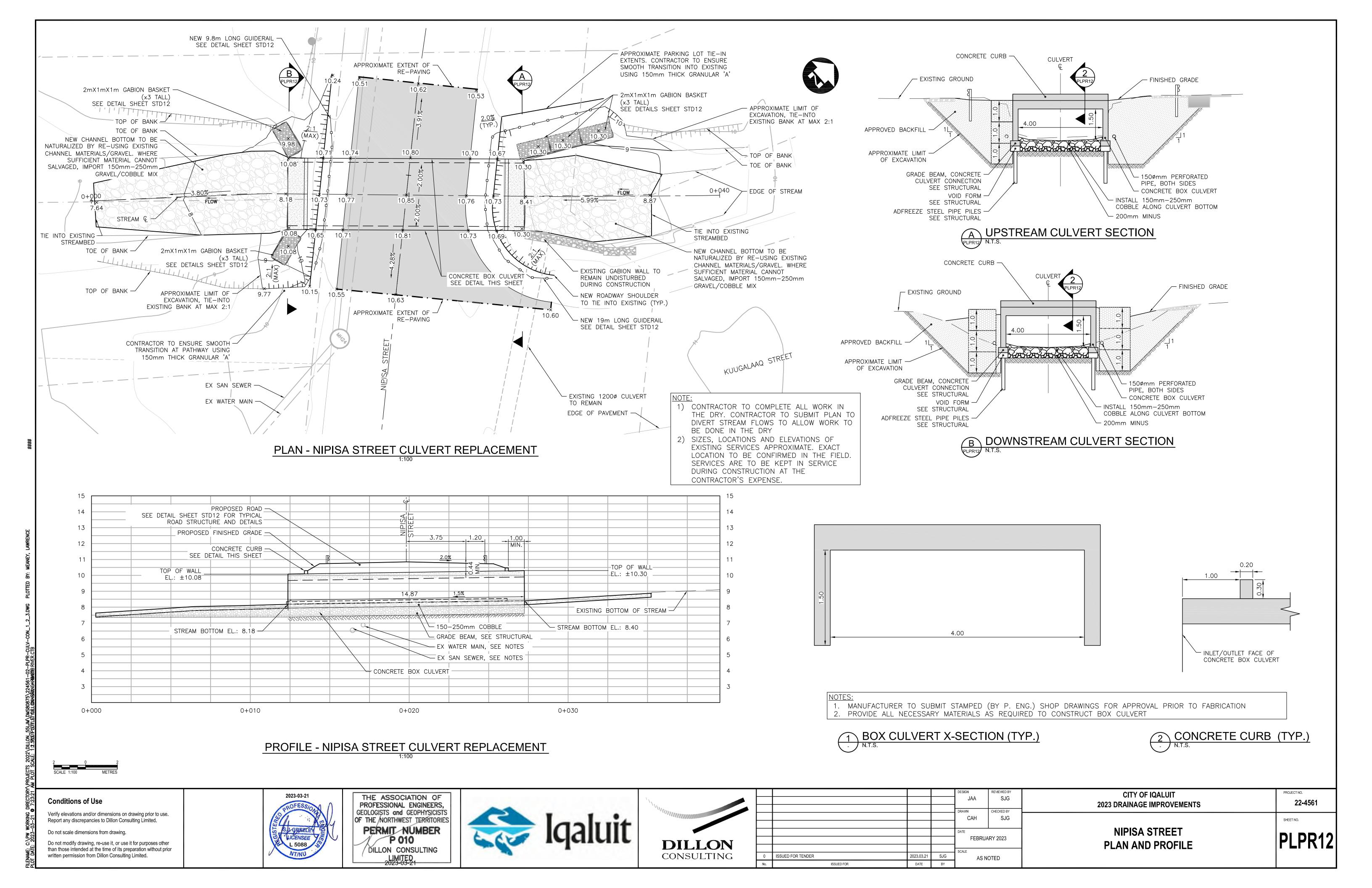
22-4561

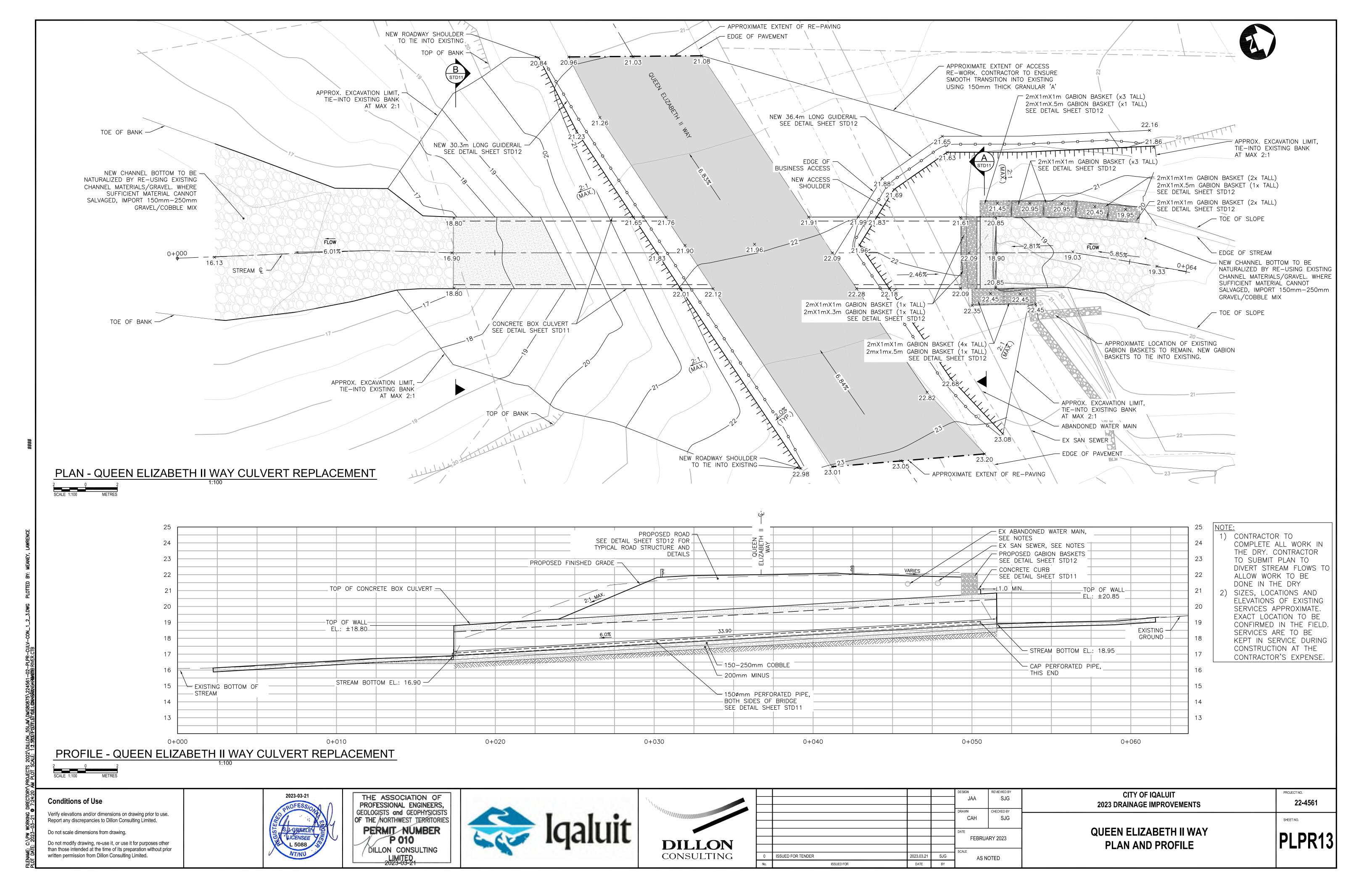


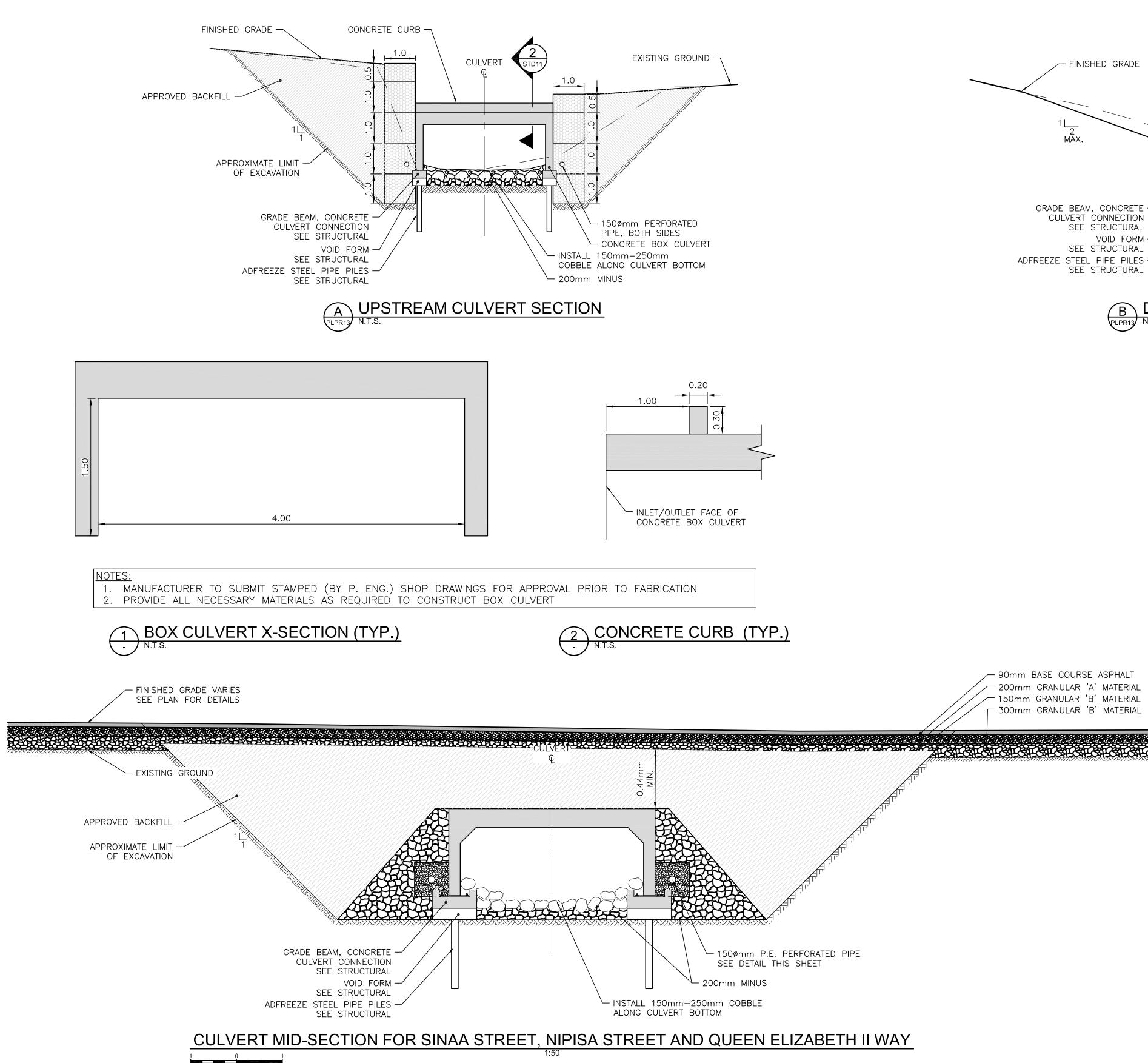


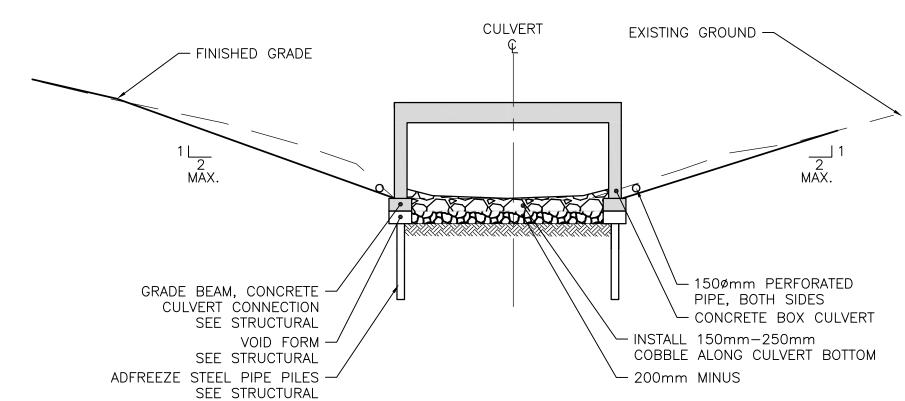




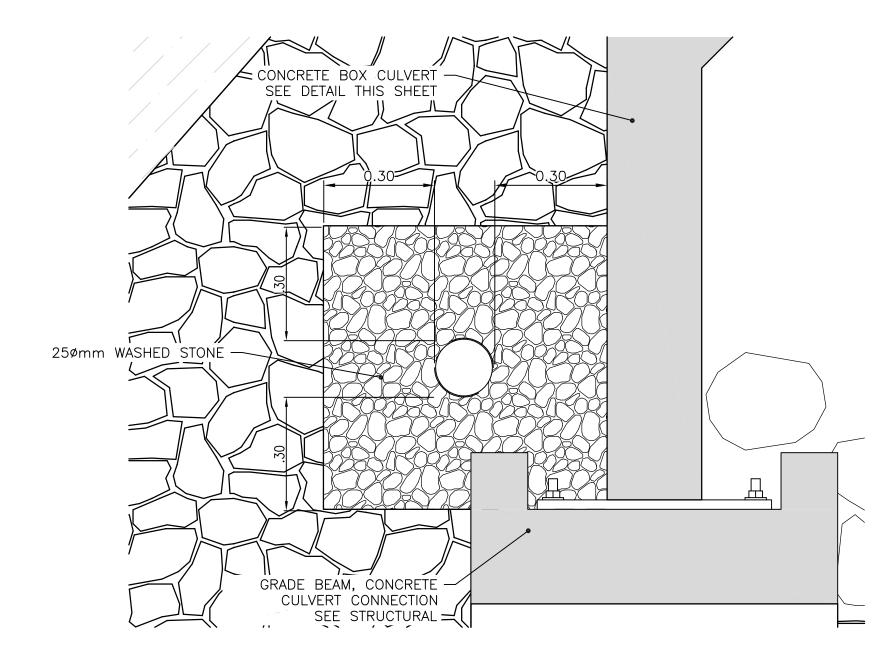












WEEPING TILE c/w PERFORATED PIPE (TYP.)

Conditions of	Use
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DILLON CONSULTING	

→ 90mm BASE COURSE ASPHALT ∠ 200mm GRANULAR 'A' MATERIAL

— 150mm GRANULAR 'B' MATERIAL

- 300mm GRANULAR 'B' MATERIAL

				DESIGN	REVIEWED BY	
				JAA	SJG	
				DRAWN	CHECKED BY	
				CAH	JAA	
				DATE		
				FEBRU <i>A</i>	ARY 2023	
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SECTIONS AND MISCELLANEOUS DETAILS

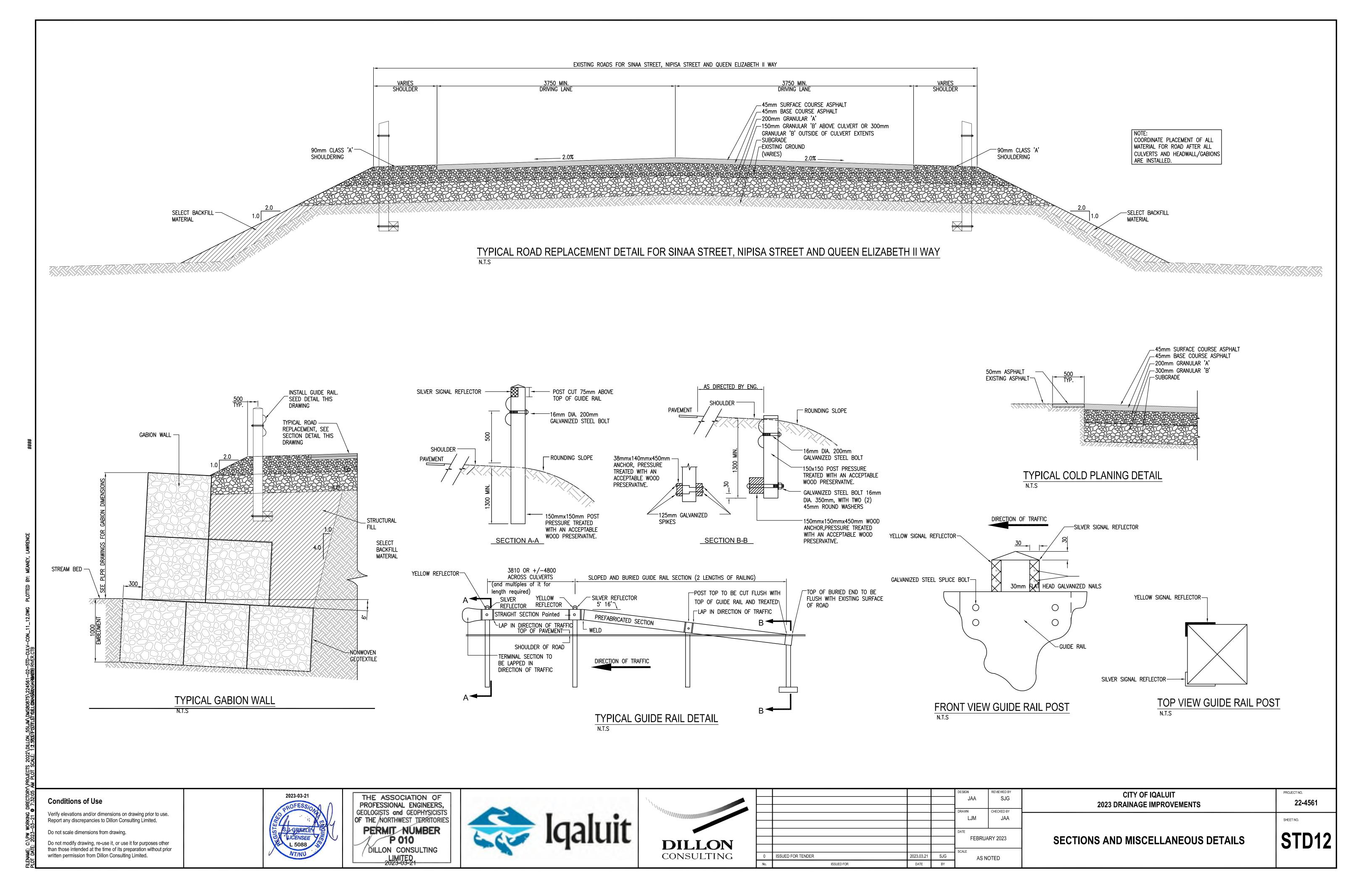
CITY OF IQALUIT

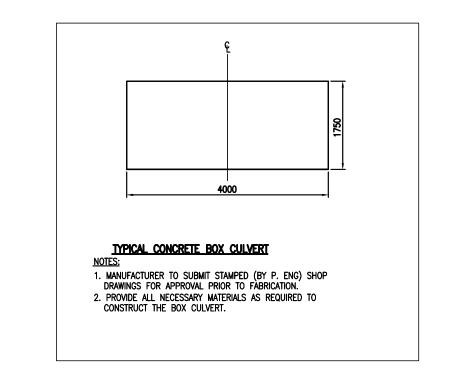
2023 DRAINAGE IMPROVEMENTS

STD11

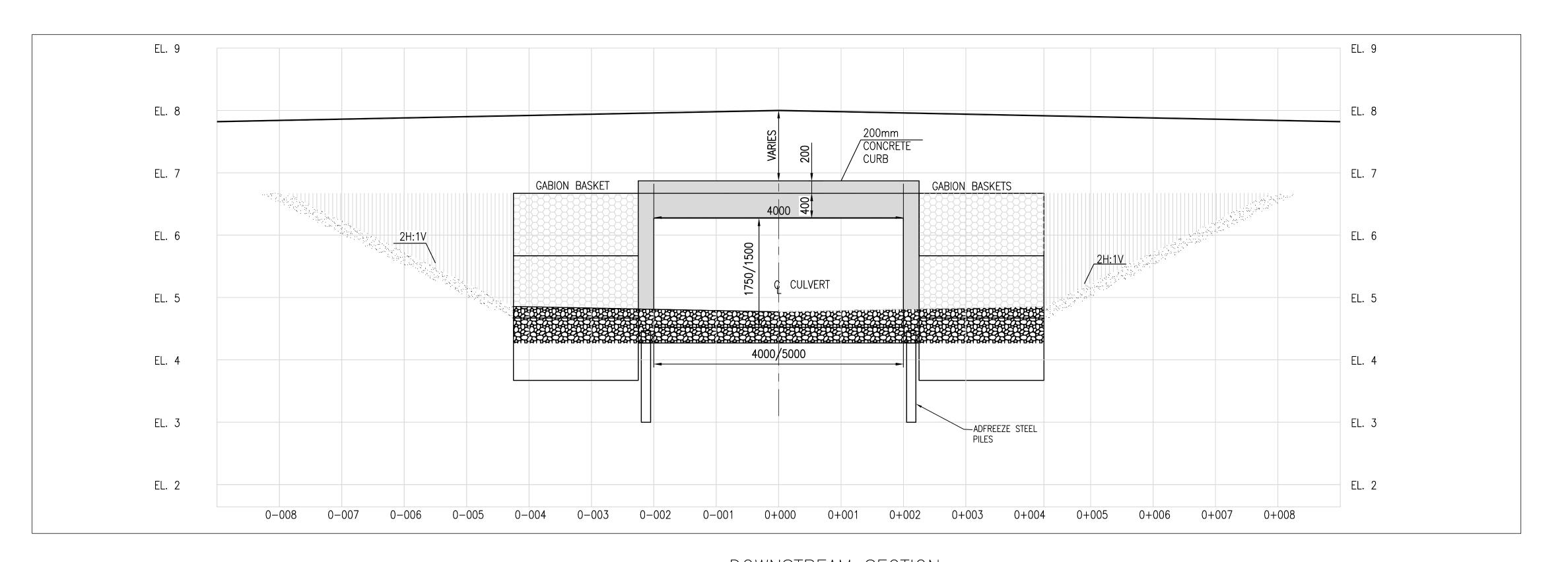
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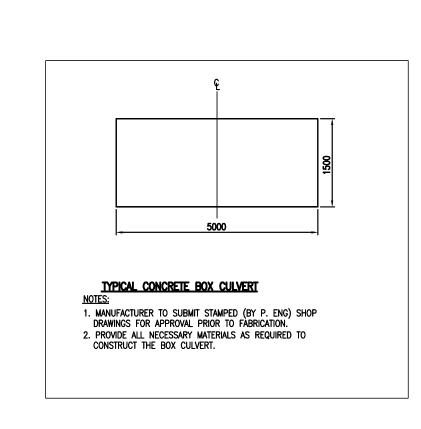
22-4561





UPSTREAM SECTION





DOWNSTREAM SECTION

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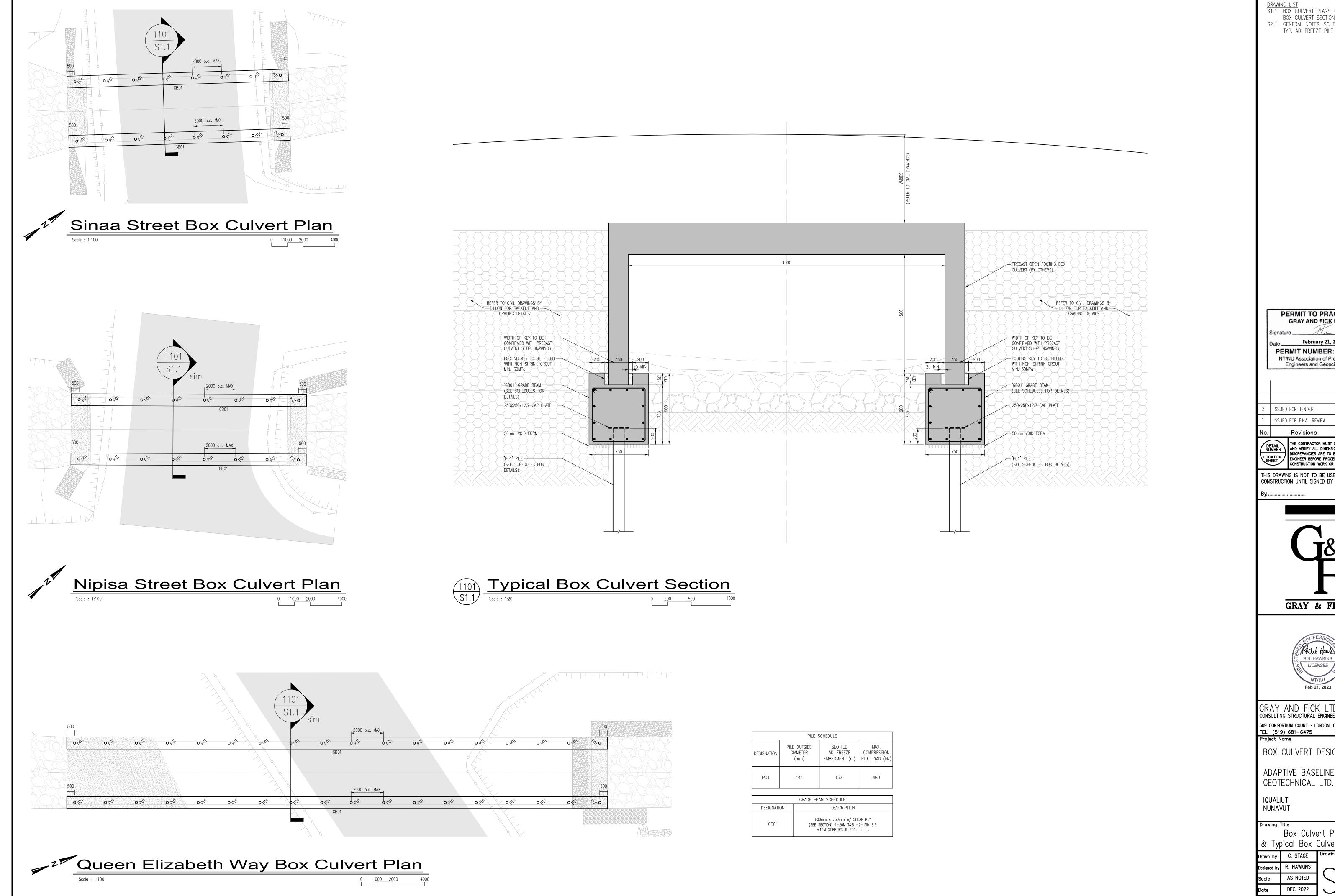
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SECTIONS AND MISCELLANEOUS DETAILS

CITY OF IQALUIT

2023 DRAINAGE IMPROVEMENTS

22-4561



DRAWING LIST S1.1 BOX CULVERT PLANS & TYPICAL BOX CULVERT SECTION S2.1 GENERAL NOTES, SCHEDULES, AND TYP. AD-FREEZE PILE DETAIL

> PERMIT TO PRACTICE GRAY AND FICK LTD.

February 21, 2023 PERMIT NUMBER: P 1395

NT/NU Association of Professional **Engineers and Geoscientists**

No.	Revisions	Ву	Date
1	ISSUED FOR FINAL REVIEW	CS	23/02/0
2	ISSUED FOR TENDER	DC	23/02/2

DETAIL NUMBER THE CONTRACTOR MUST CHECK ALL DRAWINGS AND VERIFY ALL DIMENSIONS ON THE JOB. DISCREPANCIES ARE TO BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH ANY CONSTRUCTION WORK OR SHOP FABRICATION.

THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION UNTIL SIGNED BY ENGINEER



GRAY AND FICK LTD. CONSULTING STRUCTURAL ENGINEERS 309 CONSORTIUM COURT · LONDON, ONTARIO · N6E 2S8 TEL: (519) 681-6475

BOX CULVERT DESIGN

GEOTECHNICAL LTD.

Box Culvert Plans & Typical Box Culvert Section

> C. STAGE AS NOTED DEC 2022

GENERAL NOTES: 1. DO NOT SCALE THESE DRAWINGS. CHECK ALL DIMENSIONS BY SITE MEASUREMENT PRIOR TO COMMENCING WORK. THE

- CONTRACTOR SHALL VERIFY ALL SITE CONDITIONS WITH THE STRUCTURAL DETAILS SHOWN. REPORT ANY INCONSISTENCIES PROMPTLY TO THE ENGINEER. . THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ANY EXISTING SERVICE PIPES OR CONDUITS TO
- REMAIN ACTIVE ON THE NEW SITE. 3. COMPLY WITH THE REQUIREMENTS OF THE ONTARIO BUILDING CODE AND THE ONTARIO CONSTRUCTION SAFETY ACT AND
- 4. DO NOT EXCEED THE DESIGN LOADS DURING CONSTRUCTION. PROVIDE ADEQUATE BRACING TO ENSURE STABILITY OF THE STRUCTURE DURING CONSTRUCTION. PROVIDE TEMPORARY BRACING AS REQUIRED.

5. ALL WORK TO BE DONE IN ACCORDANCE WITH 2015 NBC-1ST WRITING AND CAN/CSA-S6-14 (CHBDC).

- STRUCTURAL STEEL:

 1. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED TO CAN3.S16.1M. SUPPLY ALL NEW STRUCTURAL STEEL:

 ANCLES AND PLATES TO ROLLED BEAMS AND COLUMNS TO CSA G40.21M GRADE 350W; HOLLOW SECTIONS, CHANNELS, ANGLES AND PLATES TO CSA G40.21M GRADE 300W CLASS 'C'. SUPPLY ALL NEW BOLT FASTENERS: ASTM A325M. ANCHOR BOLTS: ASTM A307M. 2. SHOP AND SITE WELDING SHALL COMPLY WITH CSA W59 AND W48 USING E480XX ELECTRODES. WELDING SHALL BE CARRIED
- OUT BY WELDERS CERTIFIED UNDER CWB IN ACCORDANCE WITH CSA W47.1. . ALL STEEL SHALL BE CLEANED TO SSPC.SP3 MINIMUM. EXPOSED MEMBERS SHALL RECEIVE 2 COATS OF ZINC RICH PRIMER TO CGSB SPEC 1.GP.171M, REMAINDER TO RECEIVE 1 SHOP COAT OF PRIMER TO CGSB SPEC 1.GP.40M, UNLESS HOT DIPPED GALVANIZING IS NOTED. EXISTING STEEL TO BE THOROUGHLY CLEANED OF ALL FLAKING, PAINT SURFACE CORROSION
- 1. ALL CONNECTIONS SHALL USE ASTM A325M BOLTS AND CONFORM TO CAN3.S16.1M AND THE CISC HANDBOOK OF STEEL CONSTRUCTION.
- 5. SUBMIT COPIES OF SHOP DRAWINGS SHOWING ERECTION AND CONNECTION DETAILS. PROVIDE ALL TEMPORARY SHORING AND BRACING TO ENSURE THE STRUCTURE REMAINS STABLE DURING CONSTRUCTION. 5. GROUT BELOW ALL COLUMN BEARING PLATES USING DRY PACK NON-SHRINK CONCRETE GROUT WITH A MINIMUM
- COMPRESSIVE STRENGTH OF 30 MPa UNO. THE SHEAR CAPACITY OF ALL BEAM AND GIRDER CONNECTIONS SHALL NOT BE LESS THAN THE SHEAR CAPACITY OF THE SECTION ACTING AS A SIMPLE BEAM LOADED UNIFORMLY TO ITS MOMENT CAPACITY OVER THE SAME SPAN NOR LESS THAN THAT SHOWN ON THE DRAWINGS, WHICHEVER IS GREATER.

SUB-FOUNDATIONS:

- . DO NOT BURY THE PILES WITHOUT WRITTEN AUTHORIZATION FROM THE ENGINEER. 2. JOINTS MUST BE MADE WITH FULL PENETRATION 'GROOVE WELDS'. ALL WELDS TO BE PREFORMED BY A FABRICATOR CERTIFIED WITH THE 'CANADIAN WELDING BUREAU' (CWB) AS PER CSA W47.1-09(R2014) AND SHALL CONFORM TO
- REQUIREMENTS ESTABLISHED IN CSA W59-03(R2008). COMPETENCE CARDS WILL BE REQUIRED. . PILE HEADS MUST NOT BE 50mm AWAY FROM THEIR THEORETICAL LOCATION. PILE VERTICAL ALIGNMENT MUST NOT DEVIATE BY MORE THAN 2% WHEN COMPARED TO THEIR FULL LENGTH.
- 1. PRIOR TO COMMENCING PILE INSTALLATION, SUBMIT TO THE ENGINEER FOR VERIFICATION. ALL CHARACTERISTICS RELATING TO PILE DRIVING EQUIPMENTS, AS WELL AS FORMULAS USED IN PILE DRIVING DESIGN CALCULATIONS. 5. PILES ARE TO BE FILLED WITH SAND OR SLURRY.
- 6. PILE TO BE VIBRATED IN PLACE TO ENSURE A GOOD SEAT ON BEDROCK.
- 7. PILE BOND LENGTH TO BE CLEANED OF GREASE, OIL OR LOOSE MATERIAL. 8. DRILLED HOLE TO BE FREE OF DEBRIS, MUD OR WATER PRIOR TO INSTALLATION.
- 9. PILE DESIGN BASED ON GEOTECHNICAL REPORT BY CANADRILL LIMITED, TITLED 'GEOTECHNICAL INVESTIGATION 2022 DRAINAGE IMPROVEMENTS IQALUIT, NU' ISSUED NOV. 4, 2022 TO CITY OF IQALUIT.
- O. WHENEVER POSSIBLE, FULL LENGTH PILES SHALL BE PROVIDED TO ELIMINATE SPLICING DURING INSTALLATION. SHOP SPLICING SHALL BE PERFORMED USING COMPLETE JOINT PREPARATION GROOVE WELDS. 11.STEEL FOR CAP PLATES AND TIP REINFORCEMENT SHALL CONFORM TO CAN/CSA STANDARD G40.21 GRADE 300W 12. APPROVAL SHALL BE OBTAINED FROM THE ENGINEER PRIOR TO PRE-BORING, WATER JETTING OR OTHER SIMILAR

TECHNIQUES. THE CONTRACTOR MAY UTILIZE ALL NORMAL METHODS TO INSTALL PILES, INCLUDING REMOVAL OF

- OBSTRUCTIONS. SPUDDING. JETTING. PRE-DRILLING OR THE USE OF FOLLOWERS. 13. PILES DAMAGED OR BROKEN DUE TO DRIVING OR MISHANDLING WILL NOT BE ACCEPTED
- 14. PILES ARE TO BE FILLED WITH A SAND SLURRY MIX BOTH INSIDE AND OUTSIDE OF THE PILE. 15. REFER TO TYPICAL PILE DETAIL(S) FOR ADDITIONAL INSTALLATION REQUIREMENTS.
- 16. CONTRACTOR TO FOLLOW RECOMMENDATIONS FOR PILE INSTALLATIONS PROVIDED IN GEOTECHNICAL REPORT. ALL PILES THAT EXCEED THE MAXIMUM STICK UP LENGTH SPECIFIED IN PILE SCHEDULE SHALL BE REPORTED BACK TO GRAY & FICK LTD. AND EVALUATED BASED ON ACTUAL STICK UP LENGTH ON A CASE BY CASE BASIS. GRAY & FICK LTD. WILL EVALUATE AND DETERMINE IF REMEDIAL ACTION IS REQUIRED.

1. DESIGN LOADS AS PER REQUIREMENTS OF CAN/CSA-S6-14 (CANADIAN HIGHWAY BRIDGE DESIGN CODE).

. GROUT USED FOR ADFREEZE STEEL PIPE PILES INSTALLATION TO BE COLD WEATHER GROUT, 'SIKAGROUT ARCTIC-100' OR APPROVED EQUIVALENT, HAVING MINIMUM UNCONFINED COMPRESSION STRENGTH OF 30 MPa OR GREATER. 2. GROUT MIX TO BE BETWEEN 20°C TO 25°C WHEN PLACED. 3. GROUT SHOULD BE PLACED WITH AN INTEGRAL MECHANICAL MINER/PUMP WITH SHEARING ACTION.

<u>CAST-IN-PLACE CONCRETE:</u>

- . CONCRETE SHALL BE SUPPLIED IN ACCORDANCE WITH CSA A23.1 AND CSA A23.3. 2. CONCRETE SHALL BE READY—MIXED AND USE MATERIAL CONFORMING TO:
- CEMENT: CSA A3001, NORMAL TYPE GU PORTLAND CEMENT AGGREGATE: CSA A23.1
- REINFORCING STEEL: CSA G30.18 GRADE 400 (400W WHERE WELDING REQUIRED) 3. CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:
- 28-DAY COMPRESSIVE STRENGTH: 30MPa SLUMP:75mm +/- 25mm
- AIR CONTENT: 5-8% EXPOSURE CLASS: C-1
- 4. WELDED WIRE FABRIC SHALL BE PLACED IN ACCORDANCE WITH THE RSIC MANUAL. WELDED WIRE FABRIC SHALL BE CHAIRED TO PROVIDE PROPER COVER.
- 5. DEVELOPMENT LENGTHS SHALL CONFORM TO THE FOLLOWING:
- STRAIGHT BAR DEVELOPMENT: 24 1/2' STRAIGHT BAR SPLICE: 32"
- HOOKED BAR DEVELOPMENT: 9 1/2"
- S. PROTECT CONCRETE SLABS FROM PREMATURE MOISTURE LOSS OR MECHANICAL INJURY FOR MINIMUM OF 4 DAYS FOLLOWING PLACEMENT. USE CLEAR LIQUID SEAL TO ASM C309 OR WET CURING. 7. CONTRACTOR SHALL ARRANGE FOR ALL TESTING OF CONCRETE TO CSA A23.1. TEST RESULTS SHALL BE FORWARDED TO THE ENGINEER FOR REVIEW.
- 8. PLACE A 19mm CHAMFER ON ALL EXPOSED EDGES. 9. WET SETTING OF REINFORCING STEEL, DOWELS, AND ANCHOR BOLTS IS PROHIBITED.

- . PRECAST BOX CULVERT UNITS SHALL BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH CAN/CSA-S6-14 (CHBDC). . SUPPORT CONDITIONS FOR DESIGN OF THE PRECAST UNITS SHALL BE BASED UPON THE ASSUMPTIÓN THAT THE `FOUNDATIONS PROVIDE VERTICAL SUPPORT ONLY (NO RESTRAINT AGAINST HORIZONTAL MOVEMENT OR ROTATION).
- 3. THE PRECAST DESIGNER SHALL VERIFY THE REINFORCING INDICATED IS ADEQUATE TO TRANSFER REQUIRED LOADS BETWEEN 1. SHOP DRAWINGS SHALL BE SUBMITTED AND BEAR THE SIGNATURE AND STAMP OF A QUALIFIED PROFESSIONAL ENGINEER
- REGISTERED IN NUNAVUT AND THE NORTHWEST TERRITORIES. 5. THE LOAD SUPPORT REACTIONS (PER METRE LENGTH OF FOUNDATION) SHALL BE INDICATED ON THE PRECAST UNIT SHOP DRAWINGS AND WILL BE REVIEWED BY THE ENGINEER TO VERIFY THE FOUNDATION DESIGN.

	<u>S LIST</u>
@	AT
A.B.	ANCHOR BOLT
ADD'N	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
ALT.	ALTERNATE
ARCH.	ARCHITECTURAL
B/B	BACK TO BACK
BEW	BOTTOM EACH WAY
BLL	BOTTOM LOWER LEVEL
BM.	BEAM
BP	BEARING PLATE
BPL	BASE PLATE
BTB	BEAM TO BEAM BOTTOM UPPER LAYER
BUL	
CANT'L	CANTILEVER
CJ	CONTROL JOINT
CLR.	CLEAR
COL.	COLUMN
CONC.	CONCRETE
CONT.	CONTINUOUS
CXN.	CONNECTION
-DO-	DITTO
DIA. or Ø	DIAMETER
DL	DEAD LOAD
EA.	EACH
EF	EACH FACE
ELEC.	ELECTRICAL
ES	EACH SIDE
EW	EACH WAY
EX. or EXIST.	EXISTING
FDN or FN	FOUNDATION
FFE	FINISHED FLOOR ELEVATION
FLR	FLOOR
FTG	FOOTING
ga.	GAUGE
HDG	HOT DIP GALVANIZED
HEF	HORIZONTAL EACH FACE
HIF	HORIZONTAL INSIDE FACE
HOF	HORIZONTAL OUTSIDE FACE
HORIZ.	HORIZONTAL
H.P.	HIGH POINT
HSS	HOLLOW STRUCTURAL SECTION
kPa	KILOPASCALS
'L'	ANGLE
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
L.P.	LOW POINT
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MIN.	MINIMUM
NTS	NOT TO SCALE
0.C.	ON CENTRE
OWSJ	OPEN WEB STEEL JOIST
psf	POUNDS PER SQUARE FOOT
R.A.	ROOF ANCHOR
REINF.	REINFORCING
s/c	SAW CUT
	SIMILAR
eim	
sim.	SIMILAN

,	(123.456)——×	TOP	OF	FOUNDATION	WALL
	123.456 ×	TOP	OF	FOOTING	

WITH

SLAB ON GRADE

SNOW PILE UP

TOP AND BOTTOM

TOP LOWER LEVEL

TOP UPPER LEVEL

VERTICAL EACH FACE VERTICAL INSIDE FACE VERTICAL OUTSIDE FACE

UNLESS NOTED OTHERWISE

VERTICAL SLOTTED CONNECTION

FINISHED FLOOR ELEVATION

TOP EACH WAY

TYPICAL

UNDER SIDE

VERTICAL

STANDARD

7/0 or t/o | TOP OF

TIONS	S LIST	<u>AD-FREEZE GE</u>	EOTECHNICAL PARAM	ETERS PILE TABLE:			
	AT ANCHOR BOLT ADDITIONAL		DE DIAMETER	UNFACTORED ULS AD-FREEZE BOND CAPACITY (kPa)		FREEZE BOND CITY (kPa)	MINIMUM EMBEDMENT BELOW FINISHED GRADE (m)
	ABOVE FINISHED FLOOR ALTERNATE	114 OR 141	I (UNSLOTTED)	175		45	10.0
	ARCHITECTURAL	114 OR 14	41 (SLOTTED)	225		60	10.0
	BACK TO BACK BOTTOM EACH WAY BOTTOM LOWER LEVEL BEAM BEARING PLATE	2) ULS GEO 3) SLS BON	OTECHNICAL RESISTAN ND CAPACITY BASED	CE FACTOR OF 0.4 SHOU CE FACTOR OF 0.3 SHOU ON 30mm CREEP OVER ST FROST JACKING FORC	JLD BE APPLIED FO 30 YEARS	R TENSION LOADS.	
	BASE PLATE		PILE	SCHEDULE			
	BEAM TO BEAM BOTTOM UPPER LAYER CANTILEVER CONTROL JOINT	DESIGNATION	PILE OUTSIDE DIAMETER (mm)	SLOTTED AD-FREEZE EMBEDMENT (m)	MAX. COMPRESSION PILE LOAD (kN)		
	COLUMN	P01	141	15.0	480		
	CONCRETE						

DESIGNATION

GB01

GRADE BEAM SCHEDULE

DESCRIPTION

900mm x 750mm w/ SHEAR KEY

(SEE SECTION) 4-20M T&B +2-15M E.F. +10M STIRRUPS @ 250mm o.c.

	PILE CUT-OFF (SEE SECTION)	250x250x12.7 PLATE WELDED TO PIPE w/6.4 WELD ALL AROUND APPLY 2 COATS OF RED IRON OXIDE TO PILE FROM 1000 BELOW GRADE, UP DRY SAND FILL INSIDE OF PILE TO FINAL GROUND ELEVATION.
E SCHEDULE DEPTH	ACTIVE LAYER	GRADE CONT. LAYER OF POLY W/ HEAVY COLD CLIMATE GREASE, TO MINIMIZE FROST ADHESION THROUGH DESIGN ACTIVE LAYER. PERMAFROST
SEE PILE PLAN AND PILE SCHEDULE FOR EMBEDMENT DEPTH	PERMAFROST	LOW SALINE SAND AND FRESH WATER SLURRY ALL AROUND THE PILE. PILE IN PERMAFROST TO BE FREE OF PAINT, LACQUER, GREASE AND DIRT. 25 MIN. w/9.5 AGGREGATE 50 MIN. w/19 AGGREGATE

Typical Ad-Freeze Pile

1. PILE TO BE VIBRATED DOWN TO EMBEDMENT DEPTH WHILE ADDING LOW SALINE SAND AND FRESH WATER SLURRY. 2. PILE TO BE BRACED TRUE AND PLUMB IN CENTRE OF HOLE FOR 24 HOURS AFTER PLACEMENT OF LOW SALINE SAND AND FRESH WATER SLURRY.

3. PILE NOT TO BE LOADED UNTIL ADEQUATE FREEZE BACK HAS OCCURRED. CONSULT GEOTECHNICAL ENGINEER.

PERMIT TO PRACTICE GRAY AND FICK LTD. Signature_ February 21, 2023 PERMIT NUMBER: P 1395 NT/NU Association of Professional **Engineers and Geoscientists**

THE CONTRACTOR MUST CHECK ALL DRAWINGS			
10.	Revisions	Ву	Date
1	ISSUED FOR FINAL REVIEW	CS	23/02/02
2	ISSUED FOR TENDER	DC	23/02/21

DETAIL NUMBER AND VERIFY ALL DIMENSIONS ON THE JOB.
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CONSTRUCTION UNTIL SIGNED BY ENGINEER





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BOX CULVERT DESIGN

ADAPTIVE BASELINE GEOTECHNICAL LTD.

IQUALIUT NUNAVUT

Project Name

General Notes, Schedules & Tvp. Ad-Dreeze Pile Detail

C. STAGE R. HAWKINS AS NOTED DEC 2022 3738